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FOREWORD

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This document should be read with all the documents forming the Project Deed.

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REVISIONS TO PREVIOUS VERSION

This document has been revised from Specification RMS D&C B58 Edition 2 Revision 2.

All revisions to the previous version (other than minor editorial and project specific changes) are indicated by a vertical line in the margin as shown here, except when it is a new edition and the text has been extensively rewritten.
RMS SPECIFICATION D&C B58
BORRED CAST-IN-PLACE REINFORCED CONCRETE PILES
(WITH PERMANENT CASING)

1 GENERAL

1.1 SCOPE

This Specification sets out the requirements for the installation of reinforced concrete piles which are constructed by progressively driving or sinking non-structural steel casings (which form part of the Works) into the ground, excavating all material from inside the casings, placing reinforcement and then infilling the casings with concrete.

Materials and construction must conform to AS 5100.3 and this Specification.

The scope of this specification does not cover the supply and installation by driving of tubular steel piles that are driven to a resistance (refer to Specification RMS D&C B54).

1.2 STRUCTURE OF THE SPECIFICATION

This specification includes a series of annexures that detail additional requirements.

1.2.1 (Not Used)

1.2.2 Schedules of HOLD POINTS, WITNESS POINTS and Identified Records

The schedules in Annexure B58/C list the HOLD POINTS and WITNESS POINTS that must be observed. Refer to Specification RMS D&C Q6 for the definitions of HOLD POINTS and WITNESS POINTS.

The records listed in Annexure B58/C are Identified Records for the purposes of RMS D&C Q6 Annexure Q/E.

1.2.3 Planning Documents

The PROJECT QUALITY PLAN must include each of the documents and requirements listed in Annexure B58/D and must be implemented.

1.2.4 Frequency of Testing

The Inspection and Test Plan must nominate the proposed frequency of testing to verify conformity of the item, which must not be less than the frequency specified in Annexure B58/L. Where a minimum frequency is not specified, nominate an appropriate frequency. Frequency of testing must conform to the requirements of RMS D&C Q6.

You may propose to the RMS Representative a reduced minimum frequency of testing. The proposal must be supported by a statistical analysis verifying consistent process capability and product characteristics. The RMS Representative may vary or restore the specified minimum frequency of testing, either provisionally or permanently, at any time.
1.2.5 Referenced Documents

Standards, specifications and test methods are referred to in abbreviated form (e.g. AS 1234). For convenience, the full titles are given in Annexure B58/M.

1.3 DEFINITIONS

The terms “you” and “your” mean “the Contractor” and “the Contractor’s” respectively.

The following definitions apply to this Specification:

**Design Toe Level:** Reduced level (RL) of the pile toe shown on the Design Documentation drawings.

**Dynamic Analysis:** A Wave Equation Analysis of a specific blow using force and velocity measured in Dynamic Testing together with measured pile/soil parameters, to replicate the measured traces of force and velocity and subsequently determine pile resistance, distribution of resistance and pile integrity (e.g. CAPWAP, TNOWAVE).

**Dynamic Data:** The force and velocity near the head of the pile and estimates of pile resistance, Net Driving Energy, pile integrity and stresses in the pile, determined immediately using electronic equipment (e.g. PDA) during pile driving.

**Dynamic Testing:** The measuring and recording of Dynamic Data for each blow of the hammer and subsequent Dynamic Analysis of specific blows. The term is the same as the High-Strain Dynamic Testing of AS 2159.

**Integrity Testing:** A non-destructive test carried out on a concrete pile after its installation/construction to verify or examine the quality of the concrete pile shaft in terms of its continuity, density, shape and length.

**Pile Design Load:** The design ultimate axial load shown on the Design Documentation drawings for the pile.

**Piling Supervisor:** Your employee responsible for supervision and control of the piling operations.

**Test Piles:** Piles nominated on the Design Documentation drawings for load testing to confirm pile design parameters.

**Wave Equation Analysis:** A predictive computer analysis of pile driving, which can use hammer, pile and soil characteristics measured during Dynamic Testing for the determination of resistance versus Set of a pile (bearing graph) or pile driveability (e.g. GRLWEAP).

2 MATERIALS

2.1 PERMANENT STEEL CASINGS

2.1.1 Steel

Unless specified otherwise on the Design Documentation drawings, steel for permanent casings must comply with the following standards:
Bored Cast-in-place Reinforced Concrete Piles (With Permanent Casing)  D&C B58

AS 3678 Grades 250 or 350; or
AS 1548 Any grades; or
AS 1594 Grade HU 240.

Steel casings must have a wall thickness not less than that shown on the Design Documentation drawings.

2.1.2 Testing of Materials

All structural steel supplied must be manufactured under quality management systems certified to AS/NZS ISO 9001 by a third party accredited by the Joint Accreditation System of Australia and New Zealand.

Provide evidence that the materials used comply with the relevant Australian Standards and RMS specifications. A mill certificate with appropriate NATA registration from the material supplier will constitute documentary evidence of compliance.

Do not use any material or part in the Works until it has been identified with the tests prior to its use.

2.1.3 Fabrication

Unless specified otherwise, the inside diameter of the casing must be one of the following standard sizes: 450, 600, 750, 900, 1050, 1200, 1500 or 1800 mm.

Fabricated casings must be one of the following types:

(a) Casings fabricated from steel plate rolled to a circular cross-sectional shape and welded at the longitudinal joint to form segments which are then shop spliced into suitable lengths. The welds for the longitudinal joints and shop splices must be full penetration butt welds. The longitudinal joints for adjoining segments must be staggered by 90° to each other.

(b) Spirally welded tubes complying with AS 1579. The welds must be full penetration butt welds carried out using the submerged arc process from both sides of the tube.

All welding must conform to Specification RMS D&C B201.

The steel casings must have an inside diameter not less than the diameter of the pile specified.

The out-of-round (i.e. difference in measurements of two diagonals at right angles) of a casing must not exceed 1% of the internal diameter, provided that the concrete cover specified on the Design Documentation drawings is maintained.

2.2 REINFORCEMENT AND CONCRETE

Unless specified otherwise in this Specification, concrete and reinforcement for the piles must comply with Specification RMS D&C B80.

3 PILING PLANT AND EQUIPMENT

The equipment for construction must be of proven capacity to excavate the pile shaft and the socket in the founding material to the specified depth and diameter to achieve the design pile resistance, including the capability to excavate an additional 20% of the nominated pile depth if required.
Without limiting the requirements of Specification RMS D&C G22, prior to bringing any piling equipment or plant to the Site, provide drawings and calculations certified by a Chartered Professional Engineer with membership of Engineers Australia practising in the field of geotechnical engineering (or equivalent) of any working platforms or supports required to keep the piling rig stable and safe during piling operations at the Site.

An equivalent to membership of Engineers Australia would be an Engineer registered on the National Engineering Register (NER) in the general area of practice of Civil Engineering and experienced in the geotechnical assessment of the stability and safety of working platforms or supports for piling rig during piling operations.

### HOLD POINT

**Process Held:** Setting up of piling rig.

**Submission Details:**
(a) Details of the proposed piling equipment and method together with certification, including calculations, by a Chartered Professional Engineer with membership of Engineers Australia practising in the field of geotechnical engineering (or equivalent), verifying that under the proposed setting-up and site conditions, the equipment nominated will be used within its safe working capacities.

(b) Certification that pile hole set out (refer Clause 4), and coring (refer Clause 5) where required, has been completed.

**Release of Hold Point:** The Nominated Authority will consider the details and certification submitted, prior to authorising the release of the Hold Point.

### 4 SET OUT

Set out the site with adequate recovery pegs and survey markers so that the drilling and/or piling rig can be set up accurately on the location and alignment for each pile.

### 5 CORING

As required by site conditions, take 50 mm diameter cores prior to excavating the pile hole, to confirm the adequacy of the pile base and shaft.

Commence taking of the core samples at a level that is at one (1) metre above the level of the top of the rock socket shown on the Design Documentation drawings, for that pile, to a depth that is at least 3 m or 3 pile diameters, whichever is the greater, below the pile Design Toe Level.

Log the cores in accordance with AS 1726, place the cores in suitable core boxes and make them available for inspection as required.
6 EXCAVATION AND CLEANING OF PILE HOLE

6.1 GENERAL

6.1.1 Weekly Program

Submit weekly a daily program showing your scheduled work for pile hole excavation and/or reinforcement cage and concrete placement.

6.1.2 Supervision

Your Piling Supervisor must supervise and control the piling operations at all times.

During excavation of each pile, a Geotechnical Engineer, engaged by you, must also be in attendance.

The Geotechnical Engineer must be a Chartered Professional Engineer with membership of Engineers Australia practising in the field of geotechnical engineering (or equivalent). An equivalent to membership of Engineers Australia would be an Engineer registered on the National Engineering Register (NER) in the general area of practice of Civil Engineering and experienced in the geotechnical assessment of pile excavation.

Submit to the Project Verifier for acceptance details of relevant qualifications and experience of the Geotechnical Engineer.

6.1.3 Pile Installation Tolerances

Tolerances on pile installation must conform to Section 7 of AS 2159, except that the inclination tolerance for vertical piles is 1%, measured on the internal side of the casings. Demonstrate to the Project Verifier that the inclination tolerances have been achieved.

6.1.4 Environmental Requirements

Remove all excavated material from the Site, unless specified otherwise.

Collect and treat the water resulting from your piling operations in accordance with Specification RMS D&C G36.

6.1.5 Pile Hole Safety

Provide effective measures to protect each pile hole from site run-off and from loose material falling in during excavation. Make each pile hole safe with appropriate measures, including covering it with a secure lid whenever the pile is not under construction. Leave the finished top of the casing at least one metre above ground level to prevent personnel and loose material from falling into the pile hole.

6.2 CASING INSTALLATION AND PILE HOLE EXCAVATION

6.2.1 Proposed Methods

Provide in the PROJECT QUALITY PLAN the proposed methods for installation of the casing, excavating and cleaning the pile holes, and for verifying that the specified requirements have been met.
6.2.2 Casing Installation

Unless specified otherwise on the Design Documentation drawings, install the pile casings by one, or a combination, of the following methods:

(a) Driving the casing with an appropriate pile driving hammer and helmet and excavating the material from inside the casing;
(b) Sinking the casing with a drilling rig and excavating the material from inside the casing.

Drive or sink the casing through any inferior and/or hard materials, without damaging it, to bear on the founding layer.

Keep the toe of the casing far enough ahead of the excavation so that the excavation will not cause significant disturbance in the adjoining ground. Control the driving energy to prevent the casing from buckling, crumpling or being forced out of round.

Control the alignment of the casing with sufficient accuracy to ensure that the drilling head can be inserted and withdrawn without fouling against the casing.

Where splicing of casings is required, the welded connection must be full penetration butt welds over the whole cross-section. Welding must be in accordance with Specification RMS D&C B201.

6.2.3 Pile Hole Excavation

After installation of the permanent casing, excavate inside the casing past the toe of the casing and into the founding material to obtain the rock socket of the specified depth in rock of the class specified on the Design Documentation drawings.

Where necessary, use acceptable methods to prevent the pile hole excavation below the casing from collapsing.

6.2.4 Record Drilling Parameters

Record and measure the operating parameters of the drilling rig when excavating the first pile hole.

Excavate subsequent pile holes after the first pile with the same drilling rig operating parameters and work methods as that recorded during excavation of the first pile hole.

6.3 Final Pile Toe Level and Pile Hole Cleaning

6.3.1 Direction by Geotechnical Engineer

The attending Geotechnical Engineer will direct the Piling Supervisor as to when ground or rock of the class specified on the Design Documentation drawings has been reached, the extent of further excavation required to form the socket in accordance with the Design Documentation drawings, and when the pile hole and socket are considered to be clean.

6.3.2 Further Excavation

After excavation of the pile hole in accordance with Clause 6.3.1, the Project Verifier will either accept the level reached as the final pile toe level or require that excavation be continued to a lower level.
Where excavation to a lower level is required, carry out further pile hole excavation to the new level required, including further driving or sinking of the permanent casing if practicable and if necessary without damaging the casing, and again clean the pile hole in accordance with Clause 6.3.3.

### 6.3.3 Side Wall Roughening and Base Cleaning

Roughen the side walls of the rock socket to remove debris and surface smear and expose intact rock.

After roughening the side walls, clean the base of the pile so that intact rock is exposed over the pile base.

### 6.3.4 Acceptance of Pile Hole

Upon completion of excavation of the pile hole, submit to the Project Verifier quality records, signed by the Piling Supervisor, demonstrating that the pile hole has been excavated to the position, size and level shown on the Design Documentation drawings or in accordance with the directions of the attending Geotechnical Engineer, and that the soil design parameters shown on the Design Documentation drawings has been achieved.

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Following release of the Hold Point by the Nominated Authority, keep the pile hole sides and base in a clean and stable condition without contamination or softening, until the concrete is placed. Place the steel reinforcement and concrete as soon as practical following final socket cleaning, and within 24 hours of the excavation of the pile hole.

Where there is a delay of more than 6 hours between cleaning and concrete placement, undertake further cleaning prior to placement of reinforcement which is to be immediately followed by placement of concrete.

### 7 INDIRECT EXAMINATION OF PILE HOLE

To verify conformity with Clause 6.3, provide all equipment and personnel necessary for the Project Verifier or Geotechnical Engineer to indirectly examine the pile hole from top to bottom, including the socket base and sidewalls.

Provide the equipment for indirect examination of the pile hole, e.g. mirrors, probes, plumb weights, bright lights, CCTV cameras.
Verify conformity of the depth of each pile hole using a weighted tape at least at three locations. Carry out further indirect examination as required.

8 REINFORCEMENT

Fix and place the reinforcement for the piles in accordance with RMS D&C B80 and this specification.

Use templates when fabricating the reinforcement cages to ensure that the required reinforcement distribution is maintained.

The spacers attached to the cage must be of an approved type and distribution capable of providing the specified concrete cover and maintaining the cage in the specified position, over the whole length of the pile, after placement and during concreting.

Where telescopic casing is used, the cage must be centralised and that the minimum concrete cover must be not less than the specified value.

Cages for raked piles must incorporate provision for inserting and removing the tremie pipe without the risk of it being snagged.

Clean the pile hole and reinforcement cage of all loose and adhering material before and after the reinforcement cage is placed.

**HOLD POINT**

Process Held: Placing steel reinforcement cage in pile hole.

Submission Details: Verification that the pile hole is clean, and that all loose materials have been cleaned from the reinforcement cage.

Release of Hold Point: The Nominated Authority will review the submitted documents and may carry out further surveillance and audit, prior to authorising the release of the Hold Point.

**WITNESS POINT**

Process to be Witnessed: Inspection of the reinforcement cage prior to placing into the pile hole.

Submission Details: Notify the Nominated Authority at least 2 hours prior to the proposed placement of the reinforcement cage into the pile hole.

9 CONCRETE

9.1 GENERAL

At the time of concrete placement, the pile hole side walls and base must be clean and conform to the design parameters specified on the Design Documentation drawings.
Commence concreting the pile within 2 hours of the release of the Hold Point in Clause 8. At any time where the 2 hour requirement is not met, the authorised release of the Hold Point may become invalid in which case the same Hold Point will be re-imposed.

9.2 DEWATERING

Where dewatering of the pile hole is practical, place the concrete using a centrally placed rigid hopper and pipe to a depth that will ensure the concrete does not hit the reinforcing cage and segregate. Where dewatering is not practical, place the concrete using tremie methods in a continuous process from the base to the top of the pile.

Stir up any sediment in the pile hole into suspension immediately before placing the concrete.

9.3 CONCRETE SUPPLY

Unless specified otherwise in this Specification, deliver and place concrete for the piles in accordance with RMS D&C B80.

Provide a continuous supply of concrete so that each pile is concreted in one uninterrupted operation.

The tremie concrete mix must be self-compacting, with a nominated slump not less than 180 mm and not more than 220 mm and in accordance with RMS D&C B80. The tolerance on these slumps is ± 40 mm.

Verify by calculations that the working time to completely fill the pile hole with concrete is within the mix reversion time. When calculating the mix reversion time, base it on concrete at the lower limit of the permitted slump range.

9.4 CONCRETE PLACING

Where a concrete pump is used, a flexible rubber hose may be used to transport the concrete between the pump and the hopper. The hose must be long enough with sufficient support to carry the concrete to the hopper when it is raised to the highest level at the end of the concreting.

Do not discharge concrete directly from the concrete pump or its discharge hose into the pile hole; use the hopper and pipe for this purpose.

The hopper and pipe of the tremie must be clean and the joints watertight throughout. Seal the tremie pipe with a plate taped to its outlet, to prevent contamination of the first charge of concrete. Alternatively, use a suitable segregation barrier such as a vermiculite plug placed in the tremie or a greased rubber ball, immediately before concrete placement. Extend the tremie pipe to the base of the pile hole before the tremie is charged with concrete.

The concrete must be of a consistency and be placed such that pockets of air or water or ground materials are not entrapped in the concrete, and the space between the reinforcement and the sides of the pile hole are completely filled with concrete.

At the start of concreting, position the outlet of the tremie pipe at the base of the pile hole. Throughout the concreting, keep the outlet of the tremie pipe a minimum distance of two metres below the top surface of the concrete by lifting the tremie pipe as the concrete level rises until the concrete surface level is above the cut-off level.

Continue concreting until sound concrete appears a minimum of 400 mm above the required pile cut-off level, to avoid defective concrete at or below cut-off level.
9.5 **CONCRETING RECORDS**

During concreting, maintain a record of the depth of the tremie outlet and the level of the concrete in the pile hole, and the corresponding volume of concrete placed.

10 **CUT-OFF AND CLEAN-UP OF TOP OF PILE**

Carefully remove any concrete and/or casing above the cut-off level without damaging the permanent work not earlier than 24 hours after completion of placement of concrete.

Cut back the concrete of the pile so that the top of the pile embedded in the substructure is undamaged, sound and free of laitance and any loose material and has a profile with surface roughness not less than 3 mm.

The reinforcement protruding from the pile must be kept clean and protected from rusting and damage.

11 **PILE LOAD TESTING**

11.1 **GENERAL**

Where specified on the Design Documentation drawings, carry out pile load testing to confirm pile design parameters. The required pile test load is specified on the Design Documentation drawings and is dependent on the extent of pile testing as specified on the Design Documentation drawings. Carry out the tests specified on the Design Documentation drawings on the nominated test piles.

Additional performance criteria for load testing, if required, must be as specified on the Design Documentation drawings.

Perform static or high-strain dynamic testing in accordance with AS 2159, and Clause 11.2 of this Specification where applicable.

Other types of pile load testing in accordance with AS 2159 may be used.

11.2 **HIGH-STRAIN DYNAMIC TESTING**

11.2.1 **General**

Where high-strain dynamic pile testing is to be carried out, extend the pile as required to allow the attachment of transducers or similar testing instruments.

Carry out high-strain dynamic testing in accordance with this Clause and AS 2159 using an approved organisation with approved equipment using an approved dynamic testing system, with subsequent wave equation analysis or signal matching carried out using an approved computer program, all as listed in the “Lists of RMS Approved Bridge Components and Systems” at: http://www.rms.nsw.gov.au/business-industry/partners-suppliers/documents/tenders-contracts/listofapprovedbridgecomponentssystems.pdf.

Testing must not result in the allowable concrete stresses being exceeded.
11.2.2 Testing Procedure

Use the following testing procedure:

(a) Attach four bolt-on transducers to the pile at a minimum of 1.5 times the maximum pile width below the head of the pile in accordance with the requirements of the system supplier.

(b) Following the connection of the transducers to the analyser, strike the pile with sufficient energy to verify the required resistance.

Record the blow counts, measured resistance and nominal and measured driving energy.

The relationship between the nominal driving energy and blow counts determined from a dynamic test is valid only for the specific combination of hammer, helmet, cushion, pile rake, pile size, pile material and founding material.

11.2.3 Dynamic Analysis

Analyse the dynamic test results for each pile tested. Analyses must include full Dynamic Analysis using measured field parameters of the test data (e.g. CAPWAP) and resistance versus blow count curves (e.g. GRLWEAP analysis), showing a minimum of six (6) different resistances and the corresponding blow counts.

11.2.4 Report

Provide to the RMS Representative and Project Verifier two copies of a report for each pile tested including:

(a) Complete PDA (or approved equivalent) output for all blows, including driving stresses and net driving energy.

(b) CAPWAP (or approved equivalent) analyses for selected blows.

(c) GRLWEAP (or approved equivalent) output in the form of resistance versus blow count curves giving the true pile resistance for specific driving energies, using data measured during the high-strain dynamic testing.

(d) Certification that the pile has been dynamically tested in accordance with this Specification. If it is not possible for this certification to be provided due to nonconformities in the testing or the pile tested, provide instead an itemised nonconformity report together with the proposed disposition.

12 PILE INTEGRITY TESTING

Pile integrity testing must be capable to confirm that the pile is sound over its full length. Carry out integrity testing on all piles nominated on the Design Documentation drawings and assess pile integrity in accordance with AS 2159.
ANNEXURES B58/A TO B58/B – (NOT USED)

ANNEXURE B58/C – SCHEDULES OF HOLD POINTS, WITNESS POINTS AND IDENTIFIED RECORDS

Refer to Clause 1.2.2.

C1 SCHEDULE OF HOLD POINTS AND WITNESS POINTS

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</tr>
</thead>
<tbody>
<tr>
<td>3</td>
<td>Hold</td>
<td>Certification that piling equipment and method under the proposed set up and site conditions will be used within its safe working capacities.</td>
</tr>
<tr>
<td>6.3.4</td>
<td>Hold</td>
<td>Acceptance of pile hole.</td>
</tr>
<tr>
<td>8</td>
<td>Hold</td>
<td>Placing of steel reinforcement cage in pile hole.</td>
</tr>
<tr>
<td>8</td>
<td>Witness</td>
<td>Inspection of reinforcement cage prior to and during placing into pile hole.</td>
</tr>
</tbody>
</table>

C2 SCHEDULE OF IDENTIFIED RECORDS

The records listed below are Identified Records for the purposes of RMS D&C Q6 Annexure Q/E.

<table>
<thead>
<tr>
<th>Clause</th>
<th>Description of Identified Record</th>
</tr>
</thead>
<tbody>
<tr>
<td>2.1.1</td>
<td>Certification that all steels for casings incorporated into the Works conform to the applicable standards together with supporting NATA endorsed test results.</td>
</tr>
<tr>
<td>6.3.4</td>
<td>Piling records, signed by Piling Supervisor, verifying conformity of the pile hole upon completion of its excavation.</td>
</tr>
<tr>
<td>7</td>
<td>Results of pile hole examination.</td>
</tr>
<tr>
<td>9.5</td>
<td>Records of tremie outlet depth, concrete level in the pile hole and volume of concrete placed during concreting.</td>
</tr>
<tr>
<td>11.2.4</td>
<td>Pile load testing results.</td>
</tr>
<tr>
<td>12</td>
<td>Pile integrity testing results.</td>
</tr>
</tbody>
</table>
ANNEXURE B58/D – PLANNING DOCUMENTS

Refer to Clause 1.2.3.

The following documents are a summary of documents that must be included in the PROJECT QUALITY PLAN. Review the requirements of this Specification and other contract documents to determine any additional documentation requirements.

(a) Weld preparation and procedure for splicing of casings (refer to Clause 2.1.3 and 6.2.2);
(b) Equipment and methods for driving casing and excavating pile hole (refer to Clauses 3 and 6.2.1);
(c) Method for overcoming obstructions to driving of casing (refer to Clause 6.2.2);
(d) Method of sealing and cleaning base and sides of pile hole (refer to Clause 6.3.3);
(e) Method of supporting reinforcement cage (refer to Clause 8);
(f) Details of mix design for tremie concrete (refer to Clause 9.3);
(g) Method of placing concrete including method of inserting and withdrawing tremie pipe (refer to Clause 9.4).

ANNEXURES B58/E TO B58/K – (NOT USED)

ANNEXURE B58/L – FREQUENCY OF TESTING

Refer to Clause 1.2.4.

<table>
<thead>
<tr>
<th>Clause</th>
<th>Characteristic Analysed</th>
<th>Test Method</th>
<th>Minimum Frequency of Testing</th>
</tr>
</thead>
<tbody>
<tr>
<td>2.2</td>
<td>Concrete for piles</td>
<td>As detailed in RMS D&amp;C B80</td>
<td>As detailed in RMS D&amp;C B80</td>
</tr>
<tr>
<td>11</td>
<td>Pile Load Testing</td>
<td>Clause 11</td>
<td>Nominated Piles as shown on the Design Documentation drawings</td>
</tr>
<tr>
<td>12</td>
<td>Integrity Testing</td>
<td>Clause 12</td>
<td>Nominated Piles as shown on the Design Documentation drawings</td>
</tr>
</tbody>
</table>
ANNEXURE B58/M – REFERENCED DOCUMENTS

Refer to Clause 1.2.5.

**RMS Specifications**

<table>
<thead>
<tr>
<th>Specification</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>RMS D&amp;C G22</td>
<td>Work Health and Safety (Construction Work)</td>
</tr>
<tr>
<td>RMS D&amp;C G36</td>
<td>Environmental Protection</td>
</tr>
<tr>
<td>RMS D&amp;C Q6</td>
<td>Quality Management System (Type 6)</td>
</tr>
<tr>
<td>RMS D&amp;C B80</td>
<td>Concrete Work For Bridges</td>
</tr>
<tr>
<td>RMS D&amp;C B201</td>
<td>Steelwork for Bridges</td>
</tr>
</tbody>
</table>

**Australian Standards**

<table>
<thead>
<tr>
<th>Standard</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>AS 1548</td>
<td>Steel plates for boilers and pressure vessels</td>
</tr>
<tr>
<td>AS 1579</td>
<td>Arc welded steel pipes and fittings for water and waste water</td>
</tr>
<tr>
<td>AS 1594</td>
<td>Hot rolled steel flat products</td>
</tr>
<tr>
<td>AS 1726</td>
<td>Geotechnical site investigations</td>
</tr>
<tr>
<td>AS 2159</td>
<td>Piling – Design and installation</td>
</tr>
<tr>
<td>AS 3678</td>
<td>Structural steel - Hot rolled plates, floorplates and slabs</td>
</tr>
<tr>
<td>AS 5100</td>
<td>Bridge design</td>
</tr>
<tr>
<td>AS 5100.3</td>
<td>Part 3: Foundations and soil-supporting structures</td>
</tr>
</tbody>
</table>